

Appendix 1

Examples

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Appendix 1 - Miscellaneous Examples

This appendix includes a set of examples, which demonstrates some of the facilities in NovaFrame. The input files for each example is found in the “NF_Examples”-folder.

1 Traffic loads: Highway-1

This example shows the use of highway traffic loads on a small bridge.

Example folder: *Highway-1*
 Example files: *Highway-1.inp*
Highway-1 lod

The traffic load consists of two tracks. The traffic line consists of element 1, 2 and 3. There are 16 positions on each element.

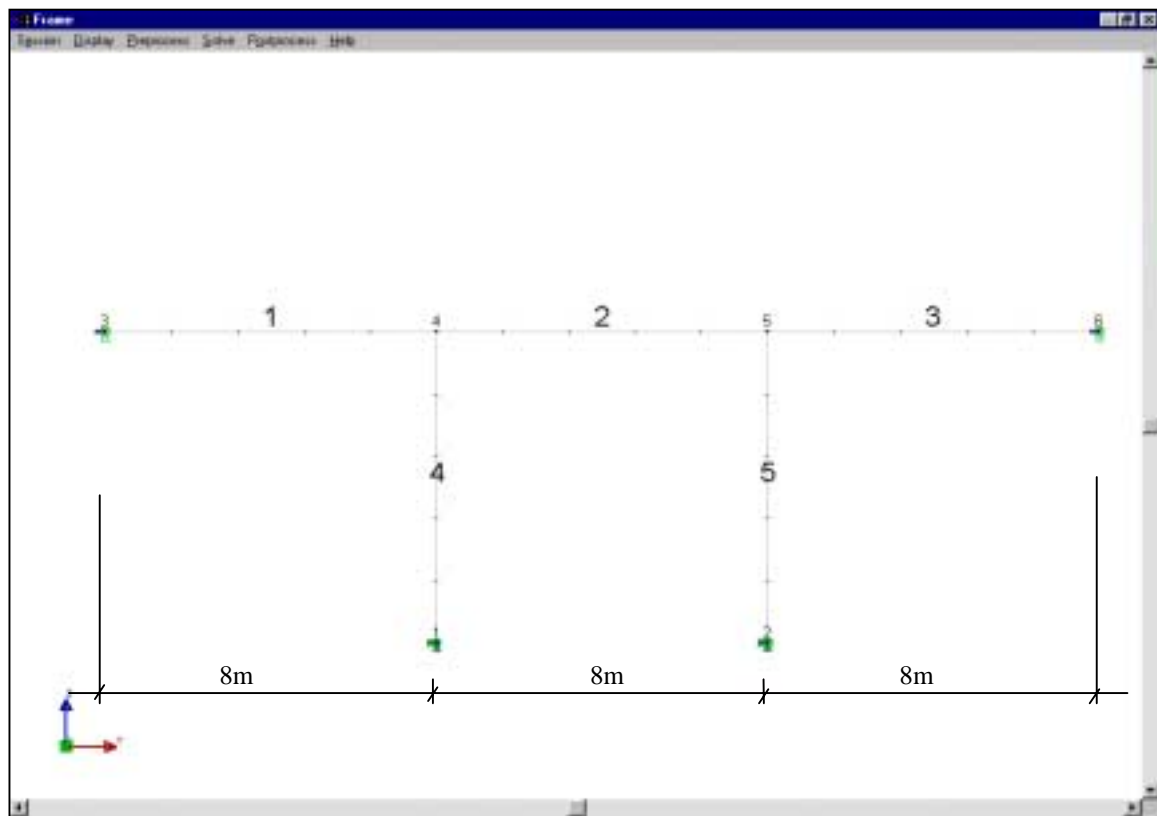


Figure 1.1 Simple Bridge

Project name: Highway-1

*** NODE INPUT ***

```
NODEINS 3 6 1 0.000 0.000 8.000 0.000 8.000 0.000
NODEINS 1 2 1 0.000 8.000 0.000 0.000 8.000 0.000
```

*** ELEMENT INPUT ***

```
ELEMINS 1 3 1 3 4 1
```

```

ELEMINS      4      4      1      1      4      1
ELEMINS      5      5      1      2      5      1

*** GEOMETRY SECTION INPUT ***
SECTINS  1  1  30000  0.0  1.000e+000  1.000e+000  1.000e+000  1.000e+000

*** ELEMENT SPECIFICATION INPUT ***
ELSPINS  1  5  1  1  0  4

*** BOUNDARY CONDITION INPUT ***
BOUNDINS      1      2      1      1      1      1      1      1      1
BOUNDINS      3      6      3      1      0      1      0      1      0

*** DESIGN SECTION INPUT ***
DESGINS      1      5      1  0.000  1.000  0.200

*** LOAD INPUT ***

*** TRAFFIC LINE INPUT ***
TRAFLINE  1  1  3  1  16  0.000

*** TRAFFIC LOAD INPUT ***
TRAFLOAD  1  1  1  1  1.000  3.000  9.0  210.0  210.0  210.0  2.500  6.000  Traffic V1
TRAFLOAD  1  2  1  1  -3.000  -1.000  9.0  210.0  210.0  210.0  2.500  6.000  Traffic V1

```

Figure 1.2 ASCII input (both files).

Element No:		1, Traffic load No:		1, X/L: 0.00			
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
1	Max Shear-L	0.00	56.91	-78.44	0.00	-0.00	621.96
3	Max Shear-N	0.00	12.69	25.82	0.00	-0.00	109.05
5	Max Torsion	0.00	39.96	-358.76	-0.00	0.00	1354.76
7	Min Shear-L	0.00	-56.91	-78.44	0.00	0.00	-621.96
9	Min Shear-N	0.00	24.20	-721.99	-0.00	0.00	882.45
11	Min Torsion	0.00	-39.96	-358.76	-0.00	-0.00	-1354.76
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-L	11111111113111121111111111141111111111111111					
2	Max Shear-L	000					
1	Max Shear-N	0000000000000000000000011111211111111113111111111141					
2	Max Shear-N	0000000000000000000000011111211111111113111111111141					
1	Max Torsion	211113111111111114111111111111111111111111111111					
2	Max Torsion	000					
1	Min Shear-L	000					
2	Min Shear-L	666666666666666676666666666666666666666666666					
1	Min Shear-N	211113111111111114100000000000000000000000000					
2	Min Shear-N	211113111111111114100000000000000000000000000					
1	Min Torsion	000					
2	Min Torsion	766668666666666666666666666666666666666666666					
Element No:		1, Traffic load No:		1, X/L: 1.00			
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
1	Max Shear-L	0.00	56.91	203.56	-132.97	455.29	-224.04
3	Max Shear-N	0.00	36.83	797.53	-344.34	294.60	-538.17
4	Max Bend-L	0.00	16.72	-8.50	137.77	133.76	78.93
6	Max Bend-N	0.00	56.91	203.56	-132.97	455.29	-224.04
5	Max Torsion	0.00	4.65	393.71	-131.77	37.16	1214.15
7	Min Shear-L	0.00	-56.91	203.56	-132.97	-455.29	224.04
9	Min Shear-N	0.00	16.39	-16.95	135.63	131.12	85.94
10	Min Bend-L	0.00	25.21	460.81	-660.32	201.71	-226.68
12	Min Bend-N	0.00	-56.91	203.56	-132.97	-455.29	224.04

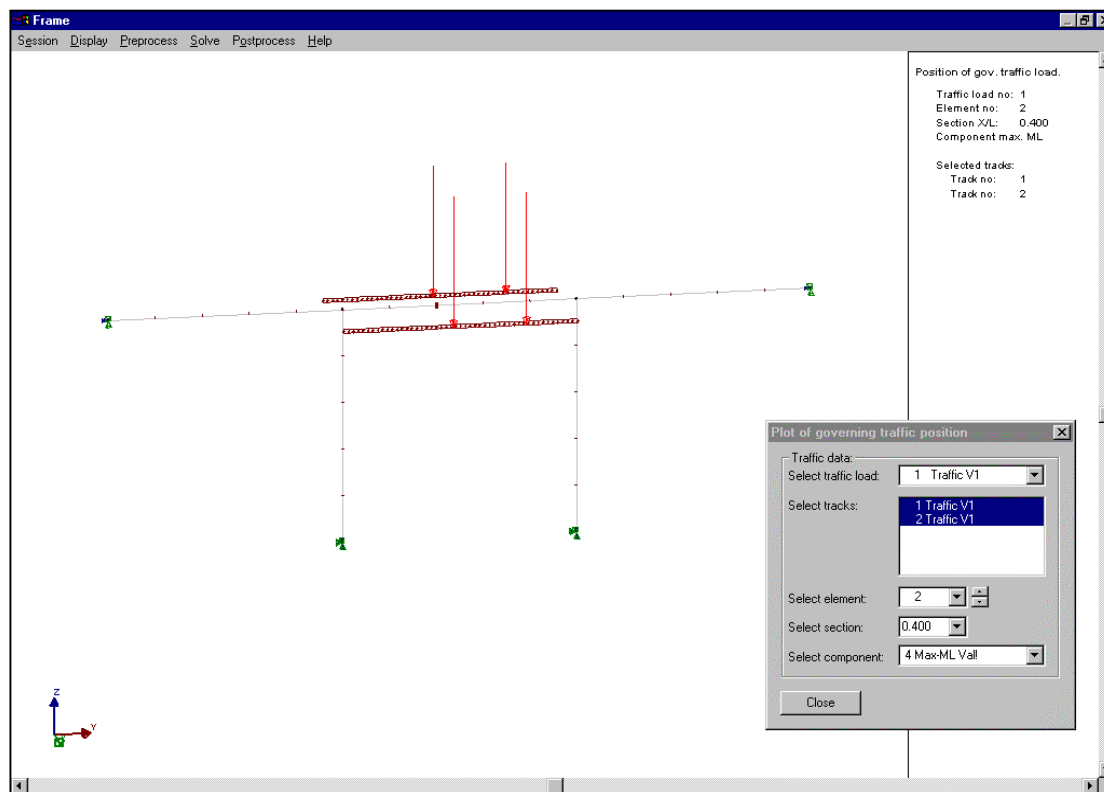


Figure 1.4 Plot of governing traffic load position for Element, 2 $X/L = 0.4$ and Max ML.

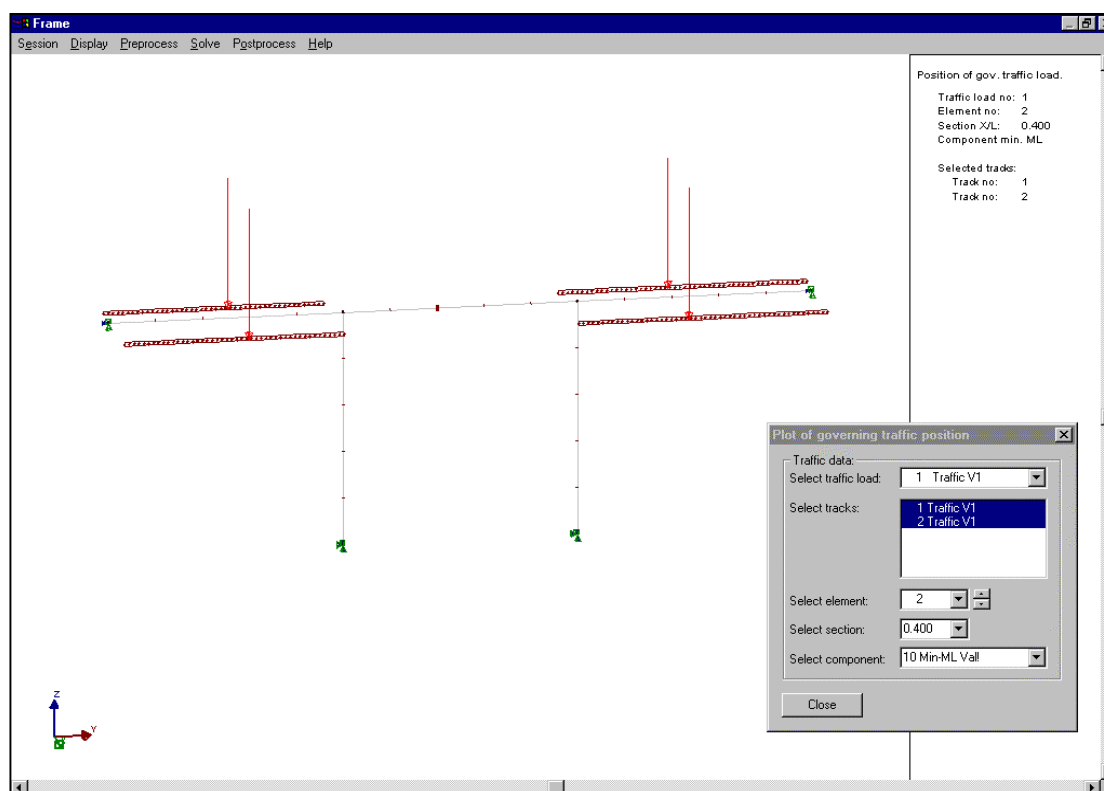


Figure 1.5 Plot of governing traffic load position for Element, 2 $X/L = 0.4$ and Min ML.

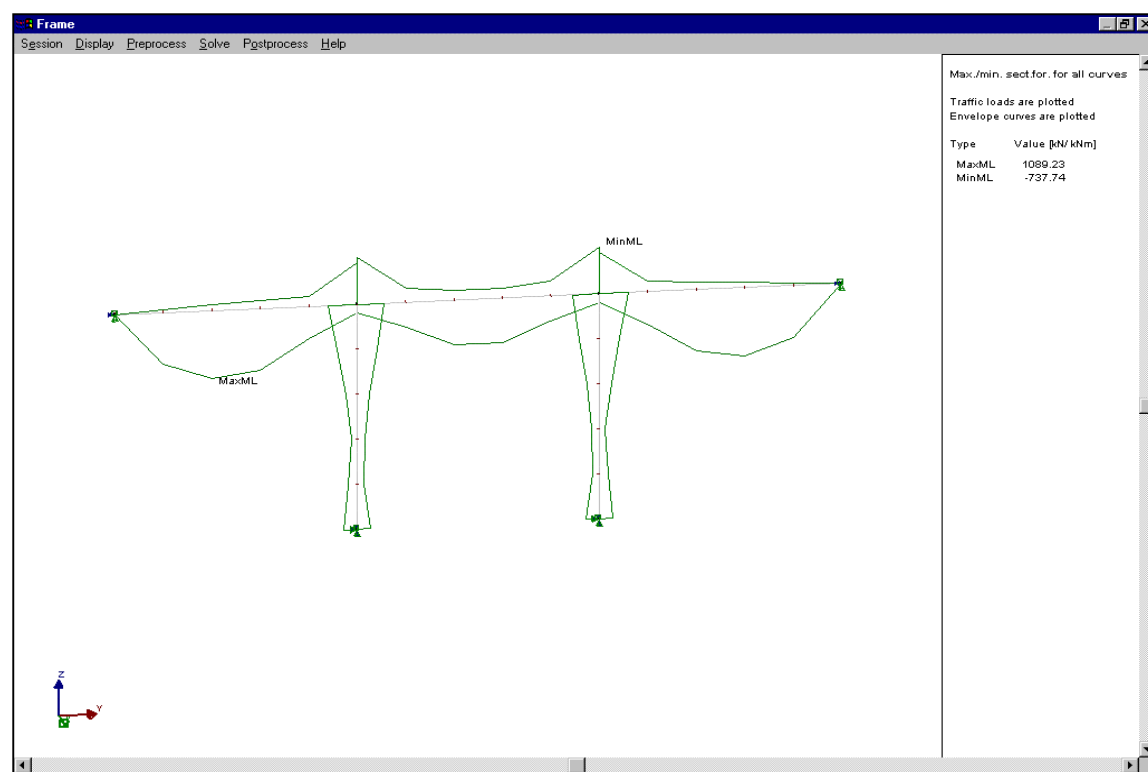


Figure 1.6 Plot of envelope curve for moment ML .

2 Traffic loads: Railway-1

This example shows the use of railway traffic loads, and how the accuracy of the method can be investigated.

Example folder: *Railway-1*
 Example files: *Railway-1.inp*
Railway-1 lod

The three types of railway loads are calculated for a bridge with two spans. Each span is one element. Three different traffic lines are defined so that accuracy of the traffic load can be evaluated. All the traffic loads consists of one single track. The input and results are presented below. Comparisons of the results are shown in figure 2.5. One should try the *Plot traffic position..*, from the **Postprocess** menu in order to view the position of the traffic loads.

```

Project name: Railway-1

*** NODE INPUT ***
NODEINS 1 2 1 0.000 0.000 0.000 0.000 40.000 0.000
NODEINS 3 3 1 0.000 100.000 0.000 0.000 0.000 0.000

*** ELEMENT INPUT ***
ELEMINS 1 2 1 1 2 1

*** GEOMETRY SECTION INPUT ***
SECTIONS 1 1 27174 0.0 1.000e+000 1.000e+000 1.000e+000 1.000e+000

*** ELEMENT SPECIFICATION INPUT ***
ELSPINS 1 2 1 1 0 4

*** BOUNDARY CONDITION INPUT ***
BOUNDINS 1 1 1 1 1 1 0 1 1
BOUNDINS 2 2 1 1 0 1 0 1 1
BOUNDINS 3 3 1 1 0 1 0 1 1

*** DESIGN SECTION INPUT ***
DESGINS 1 2 1 0.000 1.000 0.500

*** LOAD INPUT ***

*** TRAFFIC LINE INPUT ***
LineNo E1 E2 dE PosNo Name
TRAFLINE 1 1 1 0 4 0.000 PosDist =10m
TRAFLINE 1 2 2 0 6 0.000 PosDist =10m
TRAFLINE 2 1 1 0 10 0.000 PosDist =4m
TRAFLINE 2 2 2 0 15 0.000 PosDist =4m
TRAFLINE 3 1 1 0 20 0.000 PosDist =2m
TRAFLINE 3 2 2 0 30 0.000 PosDist =2m

*** TRAFFIC LOAD INPUT ***
LoadNo Tr Li Dir E-min E-max Q P1 P2 P3 distA distB Name
TRAFLOAD 100 1 1 2 0.000 0.000 80.000 250.0 0.0 0.0 1.600 0.000 UIC71-Line1
TRAFLOAD 120 1 2 2 0.000 0.000 80.000 250.0 0.0 0.0 1.600 0.000 UIC71-Line2
TRAFLOAD 140 1 3 2 0.000 0.000 80.000 250.0 0.0 0.0 1.600 0.000 UIC71-Line3
TRAFLOAD 200 1 1 3 0.000 0.000 133.000 0.0 0.0 0.0 15.000 5.300 SW/0-Line1
TRAFLOAD 220 1 2 3 0.000 0.000 133.000 0.0 0.0 0.0 15.000 5.300 SW/0-Line2
TRAFLOAD 240 1 3 3 0.000 0.000 133.000 0.0 0.0 0.0 15.000 5.300 SW/0-Line3
TRAFLOAD 300 1 1 3 0.000 0.000 150.000 0.0 0.0 0.0 25.000 7.000 SW/2-Line1
TRAFLOAD 320 1 2 3 0.000 0.000 150.000 0.0 0.0 0.0 25.000 7.000 SW/2-Line2
TRAFLOAD 340 1 3 3 0.000 0.000 150.000 0.0 0.0 0.0 25.000 7.000 SW/2-Line3

```

Figure 2.1 ASCII input, (both files).

Traffic load No: 100, Element No: 2, X/L: 0.00							
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
108	Min Shear-N	0.00	0.00	-3067.12	-29027.08	0.00	0.00
109	Min Bend-L	0.00	0.00	-3014.76	-29885.42	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Min Shear-N	1111511111					
1	Min Bend.-L	1111115111					
Traffic load No: 100, Element No: 2, X/L: 0.50							
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
102	Max Shear-N	0.00	0.00	453.99	13619.53	0.00	0.00
103	Max Bend-L	0.00	0.00	-468.99	26929.97	0.00	0.00
108	Min Shear-N	0.00	0.00	-972.74	11817.71	0.00	0.00
109	Min Bend-L	0.00	0.00	-120.16	-3604.69	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	0000115000					
1	Max Bend.-L	0000111511					
1	Min Shear-N	1111000511					
1	Min Bend.-L	1151000000					
Traffic load No: 100, Element No: 2, X/L: 1.00							
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
102	Max Shear-N	0.00	0.00	2213.37	-0.50	0.00	0.00
108	Min Shear-N	0.00	0.00	-120.16	-0.00	0.00	0.00
109	Min Bend-L	0.00	0.00	2030.94	-0.50	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	0000111115					
1	Min Shear-N	1151000000					
1	Min Bend.-L	0100151111					
Traffic load No: 120, Element No: 2, X/L: 0.00							
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
128	Min Shear-N	0.00	0.00	-3216.66	-28839.36	0.00	0.00
129	Min Bend-L	0.00	0.00	-3125.26	-30555.36	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Min Shear-N	11111111113311111111111111					
1	Min Bend.-L	11111111111111113311111111					
Traffic load No: 120, Element No: 2, X/L: 0.50							
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
122	Max Shear-N	0.00	0.00	588.74	17662.00	0.00	0.00
123	Max Bend-L	0.00	0.00	-228.06	29198.05	0.00	0.00
128	Min Shear-N	0.00	0.00	-949.91	11502.67	0.00	0.00
129	Min Bend-L	0.00	0.00	-125.42	-3762.48	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	00000000001111113300000000					
1	Max Bend.-L	00000000001111111331111111					
1	Min Shear-N	11111111110000000003311111					
1	Min Bend.-L	11111331110000000000000000					
Traffic load No: 120, Element No: 2, X/L: 1.00							

Figure 2.2 Results for element 2, UIC-71 traffic loads.

28.09.00

209	Min Bend-L	0.00	0.00	-182.87	-5486.25	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	0001111000					
1	Max Bend.-L	0000011110					
1	Min Shear-N	0000000111					
1	Min Bend.-L	1111000000					
Traffic load No: 200, Element No: 2, X/L: 1.00							

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
202	Max Shear-N	0.00	0.00	3129.19	-0.53	0.00	0.00
208	Min Shear-N	0.00	0.00	-182.87	-0.00	0.00	0.00
209	Min Bend-L	0.00	0.00	2144.63	-0.53	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	0000001111					
1	Min Shear-N	1111000000					
1	Min Bend.-L	0000011110					
Traffic load No: 220, Element No: 2, X/L: 0.00							

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
228	Min Shear-N	0.00	0.00	-3345.64	-21986.50	0.00	0.00
229	Min Bend-L	0.00	0.00	-2830.57	-25130.26	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Min Shear-N	0000000000111101111000000					
1	Min Bend.-L	0000000000001111011110000					
Traffic load No: 220, Element No: 2, X/L: 0.50							

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
222	Max Shear-N	0.00	0.00	687.42	20622.34	0.00	0.00
223	Max Bend-L	0.00	0.00	-110.32	30738.04	0.00	0.00
228	Min Shear-N	0.00	0.00	-1101.52	19090.47	0.00	0.00
229	Min Bend-L	0.00	0.00	-147.47	-4424.11	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	0000000001111011110000000					
1	Max Bend.-L	0000000000000111101111000					
1	Min Shear-N	0000000000000000000111111					
1	Min Bend.-L	0111101111000000000000000					
Traffic load No: 220, Element No: 2, X/L: 1.00							

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
222	Max Shear-N	0.00	0.00	2997.77	-0.48	0.00	0.00
228	Min Shear-N	0.00	0.00	-147.47	-0.00	0.00	0.00
229	Min Bend-L	0.00	0.00	2997.77	-0.48	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	0000000000000000011111111					
1	Min Shear-N	0111101111000000000000000					
1	Min Bend.-L	0000000000000000011111111					
Traffic load No: 240, Element No: 2, X/L: 0.00							

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
248	Min Shear-N	0.00	0.00	-3344.90	-21941.81	0.00	0.00
249	Min Bend-L	0.00	0.00	-2688.47	-25116.07	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Min Shear-N	000000000000000000001111111001111110000000000000					
1	Min Bend.-L	00000000000000000000000001111111100111111100000000					

[illegible]

Figure.2.3 Results for element 2, SW/O traffic loads.

Traffic load No:		300,	Element No:		2, X/L:	0.00		

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	
308	Min Shear-N	0.00	0.00	-5184.38	-41062.50	0.00	0.00	
309	Min Bend-L	0.00	0.00	-5063.15	-41289.06	0.00	0.00	
Traffic load position details:								
Track: Comp.:		Positions:						
1	Min Shear-N	0000111111						
1	Min Bend.-L	0001111110						
Traffic load No: 300, Element No: 2, X/L: 0.50								

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	
302	Max Shear-N	0.00	0.00	556.64	16698.77	0.00	0.00	
303	Max Bend-L	0.00	0.00	-684.38	46968.30	0.00	0.00	
308	Min Shear-N	0.00	0.00	-1422.66	24820.31	0.00	0.00	
309	Min Bend-L	0.00	0.00	-206.25	-6187.50	0.00	0.00	
Traffic load position details:								
Track: Comp.:		Positions:						
1	Max Shear-N	0111111000						
1	Max Bend.-L	0000111111						
1	Min Shear-N	0000000111						
1	Min Bend.-L	1111000000						
Traffic load No: 300, Element No: 2, X/L: 1.00								

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	
302	Max Shear-N	0.00	0.00	3815.62	-0.90	0.00	0.00	
308	Min Shear-N	0.00	0.00	-206.25	-0.00	0.00	0.00	
309	Min Bend-L	0.00	0.00	3815.62	-0.90	0.00	0.00	
Traffic load position details:								
Track: Comp.:		Positions:						
1	Max Shear-N	0000111111						

1	Min Shear-N	1111000000						
1	Min Bend.-L	0000111111						
Traffic load No: 320, Element No: 2, X/L: 0.00								
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	
328	Min Shear-N	0.00	0.00	-4809.00	-36540.00	0.00	0.00	
329	Min Bend-L	0.00	0.00	-4511.79	-36707.40	0.00	0.00	
Traffic load position details:								
Track: Comp.:		Positions:						
1	Min Shear-N	00000000001111111011111111						
1	Min Bend.-L	000000011111111011111111000						
Traffic load No: 320, Element No: 2, X/L: 0.50								
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	
322	Max Shear-N	0.00	0.00	690.69	20720.28	0.00	0.00	
323	Max Bend-L	0.00	0.00	-137.19	41484.04	0.00	0.00	
328	Min Shear-N	0.00	0.00	-1242.31	21530.60	0.00	0.00	
329	Min Bend-L	0.00	0.00	-182.25	-5467.50	0.00	0.00	
Traffic load position details:								
Track: Comp.:		Positions:						
1	Max Shear-N	00011111110111111100000000						
1	Max Bend.-L	00000000000001111111011111						
1	Min Shear-N	00000000000000000001111111						
1	Min Bend.-L	11011111110000000000000000						
Traffic load No: 320, Element No: 2, X/L: 1.00								
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	
322	Max Shear-N	0.00	0.00	3591.00	-0.84	0.00	0.00	
328	Min Shear-N	0.00	0.00	-182.25	-0.00	0.00	0.00	
329	Min Bend-L	0.00	0.00	3591.00	-0.84	0.00	0.00	
Traffic load position details:								
Track: Comp.:		Positions:						
1	Max Shear-N	00000000001111111011111111						
1	Min Shear-N	11011111110000000000000000						
1	Min Bend.-L	00000000001111111011111111						
Traffic load No: 340, Element No: 2, X/L: 0.00								
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	
348	Min Shear-N	0.00	0.00	-4601.92	-34315.45	0.00	0.00	
349	Min Bend-L	0.00	0.00	-4342.73	-34363.55	0.00	0.00	
Traffic load position details:								
Track: Comp.:		Positions:						
1	Min Shear-N	00000000000000000000111111111111100011111111111110						
1	Min Bend.-L	00000000000000000000111111111111100011111111111111						
Traffic load No: 340, Element No: 2, X/L: 0.50								
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	
342	Max Shear-N	0.00	0.00	572.37	17170.74	0.00	0.00	
343	Max Bend-L	0.00	0.00	-339.53	38413.93	0.00	0.00	
348	Min Shear-N	0.00	0.00	-1394.42	24467.46	0.00	0.00	
349	Min Bend-L	0.00	0.00	-172.83	-5184.79	0.00	0.00	
Traffic load position details:								
Track: Comp.:		Positions:						
1	Max Shear-N	000000111111111111110001111111111111000000000000000						
1	Max Bend.-L	00000000000000000000000001111111111111000111111111						
1	Min Shear-N	00000000000000000000000000000000000000011111111111100						
1	Min Bend.-L	1110001111111111111100000000000000000000000000000000000						
Traffic load No: 340, Element No: 2, X/L: 1.00								
Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion	

342	Max Shear-N	0.00	0.00	3457.27	-0.78	0.00	0.00
348	Min Shear-N	0.00	0.00	-172.83	-0.00	0.00	0.00
349	Min Bend-L	0.00	0.00	3457.27	-0.78	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	00000000000000000000111111111111000111111111111					
1	Min Shear-N	111000111111111111110000000000000000000000000000					
1	Min Bend.-L	00000000000000000000111111111111000111111111111					
Element No: 2, Traffic load No: 340, X/L: 1.00							

Lc	Comp	Axial	Shear L	Shear N	Bending L	Bending N	Torsion
341	Max Axial	0.00	0.00	0.00	0.00	0.00	0.00
340	Max Shear-L	0.00	0.00	0.00	0.00	0.00	0.00
342	Max Shear-N	0.00	0.00	3457.27	-0.78	0.00	0.00
343	Max Bend-L	0.00	0.00	0.00	0.00	0.00	0.00
345	Max Bend-N	0.00	0.00	0.00	0.00	0.00	0.00
344	Max Torsion	0.00	0.00	0.00	0.00	0.00	0.00
347	Min Axial	0.00	0.00	0.00	0.00	0.00	0.00
346	Min Shear-L	0.00	0.00	0.00	0.00	0.00	0.00
348	Min Shear-N	0.00	0.00	-172.83	-0.00	0.00	0.00
349	Min Bend-L	0.00	0.00	3457.27	-0.78	0.00	0.00
351	Min Bend-N	0.00	0.00	0.00	0.00	0.00	0.00
350	Min Torsion	0.00	0.00	0.00	0.00	0.00	0.00
Traffic load position details:							
Track: Comp.:		Positions:					
1	Max Shear-N	00000000000000000000111111111111000111111111111					
1	Max Bend.-L	00000000000000000000111111111111000111111111					
1	Min Shear-N	11100011111111111111000000000000000000000000000					
1	Min Bend.-L	00000000000000000000111111111111000111111111111					

Figure 2.4 Results for element 2, SW/2 traffic loads.

Selected points for results:

A = Element 2, X/L = 0.0

B = Element 2, $X/L = 0.5$

C = Element 2, X/L = 1.0

Load type	Load No	Pos.	A (PN-Min)	B (ML-Max)	C (PN-Max)
UIC 71	100	10	-3067.1	26930.0	2213.4
UIC 71	120	25	-3216.7	29198.0	2368.1
UIC 71	140	50	-3379.1	30894.0	2515.2
SW/0	200	10	-4026.9	37738.5	3129.2
SW/0	220	25	-3345.6	30738.0	2997.8
SW/0	240	50	-3344.9	30485.9	2669.3
SW/2	300	10	-5184.4	46968.3	3815.6
SW/2	320	25	-4809.0	41484.0	3591.0
SW/2	340	50	-4601.9	38143.9	3457.3

Figure 2.5 Comparing results for the different traffic loads

It can be concluded that the accuracy of the results will depend on the selected number of position.

3 Dynamic loads: Earthquake-1

This is an example, which demonstrates the use of earthquake analyses in NovaFrame.

Example folder: *Earthquake-1*
 Example files: *Earthquake-1.inp*
Earthquake-1 lod

Earthquake response is calculated for 30 modes. Results for the main ground acceleration in global Y- and X-direction is calculated.

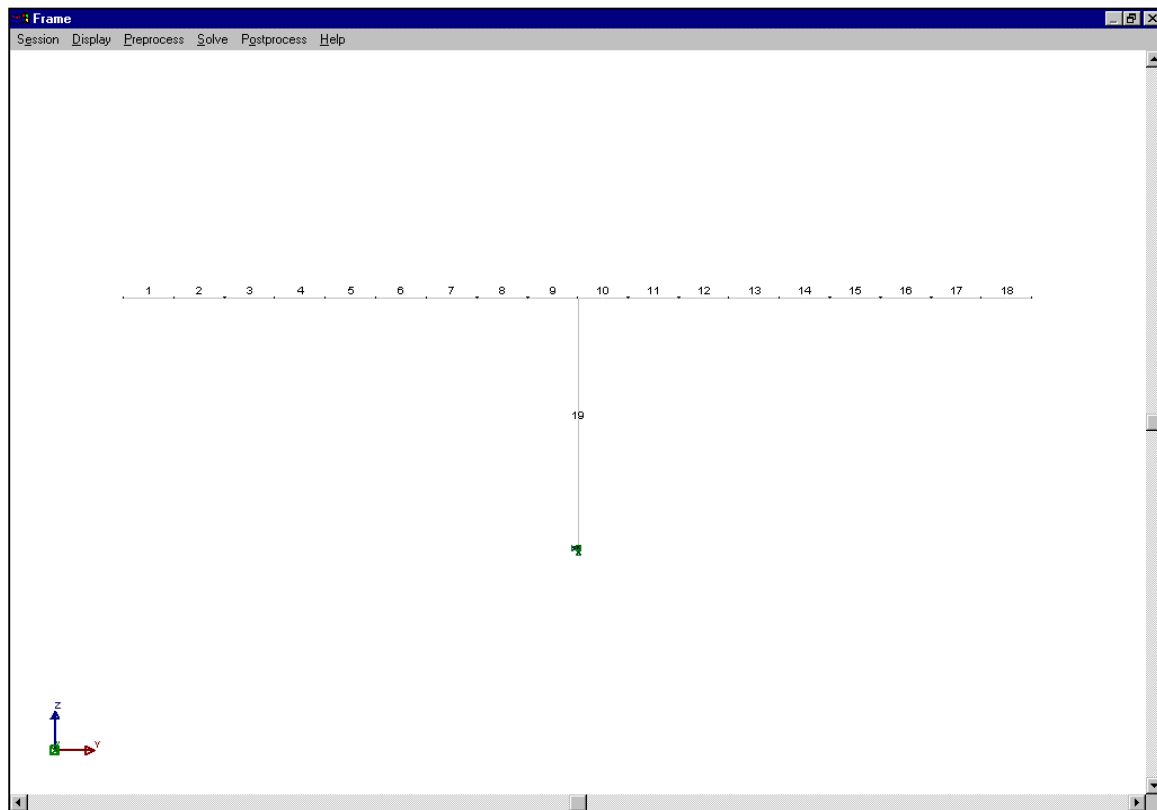


Figure 3.1 Cantilever -T.

The ASCII input is:

```
Project name: Earthquake 1

*** NODE INPUT ***
NODEINS 1 19 1 0.000 0.000 50.000 0.000 10.000 0.000
NODEINS 20 20 1 0.000 90.000 0.000 0.000 0.000 0.000

*** ELEMENT INPUT ***
ELEMINS 1 18 1 1 2 1
ELEMINS 19 19 1 20 10 1

*** GEOMETRY SECTION INPUT ***
SECTINS 1 1 30000 0.0 10 100 200 100

*** ELEMENT SPECIFICATION INPUT ***
ELSPINS 1 19 1 1 0 4
```

```

*** BOUNDARY CONDITION INPUT ***
BOUNDINS      20      20      1      1      1      1      1      1

*** ELEMENTMASS INPUT ***
ELEMMASS      1      19      1      3      0      75.000000      0.000000

*** LOAD INPUT ***

*** TRAFFIC LOAD INPUT ***
EARTHQUA      100      1      30      0      2.500      1.000      0.670      0.500      0.050 Maindir X
EARTHQUA      101      1      30      0      2.500      0.670      1.000      0.500      0.050 Maindir Y

```

Figure 3.2 ASCII input, (both files).

```

Loadcase no.:      100
Ground Accelration Ga:      2.500
Scale of Ga in X-dir:      1.000
Scale of Ga in X-dir:      0.670
Scale of Ga in X-dir:      0.500

Mode:      Freq:      SVx:      SVy:      SVz:      W-Max:      MRx:      MRy:      MRz:
1          0.169      0.047      0.032      0.024      0.021      0.000      0.177      0.000
2          0.185      0.052      0.035      0.026      0.000      0.000      0.000      0.000
3          0.289      0.081      0.054      0.040      0.064      0.930      0.000      0.000
4          0.432      0.121      0.081      0.060      0.035      0.000      0.000      0.545
5          0.601      0.168      0.113      0.084      0.028      0.000      0.807      0.000
6          0.752      0.181      0.121      0.090      0.018      0.070      0.000      0.000
7          1.978      0.181      0.121      0.090      0.000      0.000      0.000      0.000
8          2.175      0.181      0.121      0.090      0.002      0.000      0.014      0.000
9          2.506      0.181      0.121      0.090      0.005      0.000      0.000      0.241
10         3.354      0.181      0.121      0.090      0.000      0.000      0.000      0.000
11         5.539      0.181      0.121      0.090      0.002      0.000      0.000      0.185
12         5.549      0.181      0.121      0.090      0.000      0.000      0.000      0.000
13         6.136      0.181      0.121      0.090      0.000      0.000      0.000      0.000
14         6.371      0.181      0.121      0.090      0.000      0.000      0.001      0.000
15         8.269      0.145      0.097      0.073      0.000      0.000      0.000      0.000
16         8.967      0.134      0.090      0.067      0.000      0.000      0.000      0.026
17         9.799      0.123      0.082      0.061      0.000      0.000      0.000      0.000
18        12.533      0.096      0.064      0.048      0.000      0.000      0.000      0.000
19        12.789      0.094      0.063      0.047      0.000      0.000      0.000      0.000
20        15.342      0.078      0.052      0.039      0.000      0.000      0.000      0.000
21        15.559      0.077      0.052      0.039      0.000      0.000      0.000      0.001
22        16.477      0.073      0.049      0.036      0.000      0.000      0.000      0.000
23        19.519      0.062      0.041      0.031      0.000      0.000      0.000      0.000
24        21.082      0.052      0.035      0.026      0.000      0.000      0.000      0.000
25        21.340      0.051      0.034      0.025      0.000      0.000      0.000      0.000
26        24.478      0.035      0.024      0.018      0.000      0.000      0.000      0.000
27        24.572      0.035      0.023      0.018      0.000      0.000      0.000      0.000
28        26.905      0.028      0.019      0.014      0.000      0.000      0.000      0.000
29        29.018      0.023      0.015      0.011      0.000      0.000      0.000      0.000
30        31.611      0.018      0.012      0.009      0.000      0.000      0.000      0.000

SUM:      1.000      1.000      1.000

Lc  Node      Ux      Uy      Uz      Rx      Ry      Rz
      [mm]      [mm]      [mm]      [rad]      [rad]      [rad]
100   1      66.9      24.8      49.6      0.00085025      0.00095188      0.00073301
100   2      60.5      24.8      41.7      0.00084247      0.00095188      0.00073032
100   3      54.4      24.8      34.0      0.00081319      0.00095188      0.00071804
100   4      48.7      24.8      26.8      0.00075979      0.00095188      0.00068889
100   5      43.7      24.7      20.3      0.00068894      0.00095188      0.00063783
100   6      39.4      24.7      14.6      0.00060355      0.00095188      0.00056191
100   7      36.1      24.6      9.8      0.00050177      0.00095188      0.00045989
100   8      33.7      24.6      6.1      0.00038577      0.00095188      0.00033176
100   9      32.3      24.5      3.4      0.00028148      0.00095188      0.00017822
100  10      31.7      24.4      1.8      0.00030073      0.00095188      0.00000000
100  11      32.3      24.5      3.4      0.00028148      0.00095188      0.00017822

```


100	12	33.7	24.6	6.1	0.00038577	0.00095188	0.00033176	
100	13	36.1	24.6	9.8	0.00050177	0.00095188	0.00045989	
100	14	39.4	24.7	14.6	0.00060355	0.00095188	0.00056191	
100	15	43.7	24.7	20.3	0.00068894	0.00095188	0.00063783	
100	16	48.7	24.8	26.8	0.00075979	0.00095188	0.00068889	
100	17	54.4	24.8	34.0	0.00081319	0.00095188	0.00071804	
100	18	60.5	24.8	41.7	0.00084247	0.00095188	0.00073032	
100	19	66.9	24.8	49.6	0.00085025	0.00095188	0.00073301	
100	20	0.0	0.0	0.0	0.00000000	0.00000000	0.00000000	
Loadcase no.:				101				
Ground Accelration Ga:				2.500				
Scale of Ga in X-dir:				0.670				
Scale of Ga in X-dir:				1.000				
Scale of Ga in X-dir:				0.500				
Mode:	Freq:	SVx:	SVy:	SVz:	W-Max:	MRx:	MRy:	MRz:
1	0.169	0.032	0.047	0.024	0.032	0.000	0.177	0.000
2	0.185	0.035	0.052	0.026	0.000	0.000	0.000	0.000
3	0.289	0.054	0.081	0.040	0.043	0.930	0.000	0.000
4	0.432	0.081	0.121	0.060	0.035	0.000	0.000	0.545
5	0.601	0.113	0.168	0.084	0.041	0.000	0.807	0.000
6	0.752	0.121	0.181	0.090	0.012	0.070	0.000	0.000
7	1.978	0.121	0.181	0.090	0.000	0.000	0.000	0.000
8	2.175	0.121	0.181	0.090	0.003	0.000	0.014	0.000
9	2.506	0.121	0.181	0.090	0.005	0.000	0.000	0.241
10	3.354	0.121	0.181	0.090	0.000	0.000	0.000	0.000
11	5.539	0.121	0.181	0.090	0.002	0.000	0.000	0.185
12	5.549	0.121	0.181	0.090	0.000	0.000	0.000	0.000
13	6.136	0.121	0.181	0.090	0.000	0.000	0.000	0.000
14	6.371	0.121	0.181	0.090	0.000	0.000	0.001	0.000
15	8.269	0.097	0.145	0.073	0.000	0.000	0.000	0.000
16	8.967	0.090	0.134	0.067	0.000	0.000	0.000	0.026
17	9.799	0.082	0.123	0.061	0.000	0.000	0.000	0.000
18	12.533	0.064	0.096	0.048	0.000	0.000	0.000	0.000
19	12.789	0.063	0.094	0.047	0.000	0.000	0.000	0.000
20	15.342	0.052	0.078	0.039	0.000	0.000	0.000	0.000
21	15.559	0.052	0.077	0.039	0.000	0.000	0.000	0.001
22	16.477	0.049	0.073	0.036	0.000	0.000	0.000	0.000
23	19.519	0.041	0.062	0.031	0.000	0.000	0.000	0.000
24	21.082	0.035	0.052	0.026	0.000	0.000	0.000	0.000
25	21.340	0.034	0.051	0.025	0.000	0.000	0.000	0.000
26	24.478	0.024	0.035	0.018	0.000	0.000	0.000	0.000
27	24.572	0.023	0.035	0.018	0.000	0.000	0.000	0.000
28	26.905	0.019	0.028	0.014	0.000	0.000	0.000	0.000
29	29.018	0.015	0.023	0.011	0.000	0.000	0.000	0.000
30	31.611	0.012	0.018	0.009	0.000	0.000	0.000	0.000
SUM:						1.000	1.000	1.000
Lc	Node	Ux	Uy	Uz	Rx	Ry	Rz	
		[mm]	[mm]	[mm]	[rad]	[rad]	[rad]	
101	1	44.8	37.0	63.0	0.00106708	0.00063776	0.00049112	
101	2	40.5	37.0	52.9	0.00105907	0.00063776	0.00048932	
101	3	36.4	37.0	43.1	0.00102767	0.00063776	0.00048109	
101	4	32.6	36.9	33.9	0.00096656	0.00063776	0.00046155	
101	5	29.3	36.9	25.5	0.00087869	0.00063776	0.00042734	
101	6	26.4	36.8	18.3	0.00076602	0.00063776	0.00037648	
101	7	24.2	36.8	12.4	0.00062979	0.00063776	0.00030812	
101	8	22.6	36.7	7.9	0.00048156	0.00063776	0.00022228	
101	9	21.6	36.6	4.4	0.00037517	0.00063776	0.00011941	
101	10	21.3	36.5	1.8	0.00044885	0.00063776	0.00000000	
101	11	21.6	36.6	4.4	0.00037517	0.00063776	0.00011941	
101	12	22.6	36.7	7.9	0.00048156	0.00063776	0.00022228	
101	13	24.2	36.8	12.4	0.00062979	0.00063776	0.00030812	
101	14	26.4	36.8	18.3	0.00076602	0.00063776	0.00037648	
101	15	29.3	36.9	25.5	0.00087869	0.00063776	0.00042734	
101	16	32.6	36.9	33.9	0.00096656	0.00063776	0.00046155	
101	17	36.4	37.0	43.1	0.00102767	0.00063776	0.00048109	
101	18	40.5	37.0	52.9	0.00105907	0.00063776	0.00048932	
101	19	44.8	37.0	63.0	0.00106708	0.00063776	0.00049112	
101	20	0.0	0.0	0.0	0.00000000	0.00000000	0.00000000	

Figure 3.3 Results for earthquake analysis.

The results above give detailed descriptions of the response for each mode. The results are obtained by selecting listing of displacements in the **List results** dialog. Select sort order by loadcase in order to get the details for each loadcase.

Note the sum of MRX, MRY and MRZ. This sum gives a check on whether or not the number of modes included in the calculations is sufficient. When the sum is 1.00, then all significant modes are included. According to NPD this sum must at least be 0.9.

4 Axial force: Axial-1

This is an example, which demonstrates the methods for specifying the axial force magnitude.

Example folder: *Axial-1*
 Example files: *Axial-1.inp*
Axial-1.lod
Axial-1.mod

The column consists of one element. There are five identical (sub-) models defined each consisting of one single element.

The axial force magnitude for model 2, 3, 4 and 5 is specified by using the available model modification commands. The initial axial force definition is given in the **AXIAL** card in the *Preprocess.. Loads..* menu.

```

Project name: Axial-1

*** NODE INPUT ***
NODEINS 1 2 1 0.000 0.000 0.000 0.000 0.000 10.000

*** ELEMENT INPUT ***
ELEMINS 1 1 1 1 2 1

*** GEOMETRY SECTION INPUT ***
SECTINS 1 1 30000 0.2 1.000e+000 1.000e+000 1.000e+000 1.000e+000

*** ELEMENT SPECIFICATION INPUT ***
ELSPINS 1 1 1 1 0 4

*** BOUNDARY CONDITION INPUT ***
BOUNDINS 1 1 1 1 1 1 1 1 1

*** DESIGN SECTION INPUT ***
DESGINS 1 1 1 0.000 1.000 0.500

*** LOAD INPUT ***
LOADINS 1 1 1 1 4 3 -370000.0 10.000 0.000 0.000
LOADINS 11 1 1 1 4 2 100.0 10.000 0.000 0.000
LOADINS 12 1 1 1 4 2 100.0 10.000 0.000 0.000
LOADINS 13 1 1 1 4 2 100.0 10.000 0.000 0.000
LOADINS 14 1 1 1 4 2 100.0 10.000 0.000 0.000
LOADINS 15 1 1 1 4 2 100.0 10.000 0.000 0.000

*** AXIALFORCE LOAD INPUT ***
AXIAL 1 1 1 2 1 0.5

*** SUBMODELS INPUT ***
MODEL 1 0 3 0 0 0 Model 1
MODEL 2 0 3 0 0 0 Model 2
MODEL 3 0 3 0 0 0 Model 3
MODEL 4 0 3 0 0 0 Model 4
MODEL 5 0 3 0 0 0 Model 5

*** SUBMODEL MODIFICATION INPUT ***
MODMODIF 2 30 1 0 0 1 0.700 0 0.000 0 0.000
MODMODIF 3 31 1 0 0 1 0.900 0 0.000 0 0.000
MODMODIF 4 32 1 1 0 -400000.0
MODMODIF 5 33

*** CALCULATION GROUP INPUT ***

```

```

CALCGRP 1 1 11 11 1
CALCGRP 2 1 12 12 1
CALCGRP 3 1 13 13 1
CALCGRP 4 1 14 14 1
CALCGRP 5 1 15 15 1
CALCGRP 10 1 1 1 1

*** ANALYSES INPUT ***
ANALYSIS 1 1 1 0 0.000
ANALYSIS 2 2 2 0 0.000
ANALYSIS 3 3 3 0 0.000
ANALYSIS 4 4 4 0 0.000
ANALYSIS 5 5 5 0 0.000
ANALYSIS 10 5 10 0 0.000
ANALYSIS 11 5 5 0 0.000

```

Figure 4.1 ASCII input files (all files).

Node	Lc	Ux [mm]	Uy [mm]	Uz [mm]	Rx [rad]	Ry [rad]	Rz [rad]
2	1	0.0	0.0	-123.3	0.00000000	0.00000000	0.00000000
2	11	0.0	1.5	0.0	-0.00022366	0.00000000	0.00000000
2	12	0.0	2.7	0.0	-0.00042108	0.00000000	0.00000000
2	13	0.0	2.0	0.0	-0.00030611	0.00000000	0.00000000
2	14	0.0	2.4	0.0	-0.00036667	0.00000000	0.00000000
2	15	0.0	1.5	0.0	-0.00022366	0.00000000	0.00000000

Figure 4.2 Results, displacement top of column.

Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
1	1	0.00	-370000.00	0.00	0.00	0.00	0.00	0.00
1	1	0.50	-370000.00	0.00	0.00	0.00	0.00	0.00
1	1	1.00	0.00	0.00	0.00	0.00	0.00	0.00
Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
11	1	0.00	0.00	0.00	-100.00	-1272.98	0.00	0.00
11	1	0.50	0.00	0.00	-100.00	-772.98	0.00	0.00
11	1	1.00	0.00	0.00	0.00	-272.99	0.00	0.00
Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
12	1	0.00	0.00	0.00	-100.00	-2214.87	0.00	0.00
12	1	0.50	0.00	0.00	-100.00	-1714.87	0.00	0.00
12	1	1.00	0.00	0.00	0.00	-1214.88	0.00	0.00
Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
13	1	0.00	0.00	0.00	-100.00	-1666.75	0.00	0.00
13	1	0.50	0.00	0.00	-100.00	-1166.75	0.00	0.00
13	1	1.00	0.00	0.00	0.00	-666.76	0.00	0.00
Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
14	1	0.00	0.00	0.00	-100.00	-1955.56	0.00	0.00
14	1	0.50	0.00	0.00	-100.00	-1455.56	0.00	0.00
14	1	1.00	0.00	0.00	-0.00	-955.57	0.00	0.00
Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
15	1	0.00	0.00	0.00	-100.00	-1272.98	0.00	0.00
15	1	0.50	0.00	0.00	-100.00	-772.98	0.00	0.00
15	1	1.00	0.00	0.00	0.00	-272.99	0.00	0.00

Figure 4.3 Results, section forces.

Magnitude of axial forces in the different analyses/models and how the values are obtained:

Analysis	Axial force:	Obtained by:
1	-185000 kN	From AXIAL-definition, found by pre-analysis.
2	-444000 kN	From AXIAL-definition, + addition by using model modification cards; $(0.5 + 0.7) * -370000$. Value of -370000 is found by pre-analysis.
3	-330000 kN	Axial force is defined by model modification only: $0.9 * -370000$. Value of -370000 is found by pre-analysis.
4	-400000 kN	Axial force is defined by model modification, value directly
5	0	From AXIAL-definition. Axial force is to be loaded from database according to model modification card. Load of axial forces from database fails. This can be viewed by pressing the “View errors and warnings”-button in the Solve Analyses dialog. (No results for loadcase 1)
10	0	Loadcase 1 is analyzed.
11	-185000 kN	From AXIAL-definition. Axial force is to be loaded from database according to model modification card. Load of axial forces from database gives: $-370000 * 0.5$.

Figure 4.4 Analyses and axial force levels.

The axial force magnitude, which is defined for each model can, when the analyses are performed and saved to the database, be viewed in the Novaframe document. The calculated axial force magnitudes are presented for each model.

Note that the moments in the column includes the second order moment generated by the axial force and the deformation.

5 Creep loads: Creep-1

This example gives a short description of the use of creep capabilities in NovaFrame.

Example folder: *Creep-1*
 Example files: *Creep-1.inp*
Creep-1.lod
Creep-1.mod

The structure consists of two cantilevers, which are built simultaneously on a fixed framework. After 30 days the framework is removed and the self-weight is activated on the cantilevers. The cantilever remains like this for 60 days until they, at the age of 90 days, are connected. At the age of 9999 days the cantilevers are again evaluated after being connected for 9909 days. This approach is somewhat theoretic but it simulates a simplified approach relevant for cantilever bridges.

The “Bridge” is symmetric, hence only the left cantilever is evaluated in the calculations. The ASCII Input is shown in figure 5.2. It is required to define two static models in order to perform the calculations at the different construction stages.

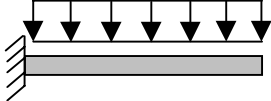
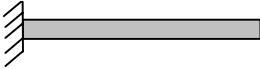

Loadcase	Model	Analysis/phase
1		Self weight applied, Model 1 Analysis 1 T = 30 [days]
2		Creep deformation, Model 1 Analysis 2 T = 30 – 90 [days]
3		Connected Creep redistribution, Model 2 Analysis 3 T = 90 – 9999 [days]

Figure 5.1 Model and analyses.

```

Project name: Creep-1

*** NODE INPUT ***
NODEINS  1  2  1  0.000  0.000  0.000  0.000  100.000  0.000

*** ELEMENT INPUT ***
ELEMINS   1   1   1   1   2   1

*** GEOMETRY SECTION INPUT ***
SECTINS  1  4  30000  0.2  10.000  10.000  1.000  1.000

*** ELEMENT SPECIFICATION INPUT ***
ELSPINS  1  1  1  1  0  4

*** BOUNDARY CONDITION INPUT ***
BOUNDINS   1       1       1       1       1       1       1       1

*** DESIGN SECTION INPUT ***
DESGINS    1       1       1  0.000  1.000  0.200
  
```

```

*** LOAD INPUT ***
LOADINS      1      1      1      1      3      3 -24.000 0.0  0.0  0.0 Self weight

*** CREEP COMBINATION INPUT ***
CREEPCMB    10      1      1.000

*** CREEP LOAD INPUT ***
CREEPLC     2      1      1      1      10      2  0.538 0.000 0.000 Creep Cant
CREEPLC     3      1      1      1      10      2  0.833 0.000 0.000 Creep Conn.

*** SUBMODELS INPUT ***
MODEL 1 0 3 0 0 0 Free End 2
MODEL 2 0 3 0 0 0 Fixed End 2

*** SUBMODEL MODIFICATION INPUT ***
MODMODIF 2 1 2 2 1 100111

*** CALCULATION GROUP INPUT ***
CALCGRP 1 1 1 1 1 Self weight canteliver
CALCGRP 2 3 2 2 1 Creep canteliver
CALCGRP 3 3 3 3 1 Creep connected

*** ANALYSES INPUT ***
ANALYSIS 1 1 1 0 0.000 Self weight
ANALYSIS 2 1 2 0 0.000 Creep canteliver
ANALYSIS 3 2 3 0 0.000 Creep connected

```

Figure 5.2 ASCII input (All files).

Node	Lc	Ux [mm]	Uy [mm]	Uz [mm]	Rx [rad]	Ry [rad]	Rz [rad]
2	1	0.0	0.0	-20.3	-0.00027100	0.00000000	0.00000000
2	2	0.0	0.0	-11.1	-0.00014872	0.00000000	0.00000000
2	3	0.0	0.0	-5.6	-0.00000000	0.00000000	0.00000000

Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
1	1	0.00	0.00	0.00	-2400.00	-120000.00	0.00	0.00
1	1	0.20	0.00	0.00	-1920.00	-76800.05	0.00	0.00
1	1	0.40	0.00	0.00	-1440.00	-43200.10	0.00	0.00
1	1	0.60	0.00	0.00	-960.00	-19200.14	0.00	0.00
1	1	0.80	0.00	0.00	-480.00	-4800.19	0.00	0.00
1	1	1.00	0.00	0.00	0.00	-0.24	0.00	0.00

Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
2	1	0.00	0.00	0.00	0.00	-0.00	0.00	0.00
2	1	0.20	0.00	0.00	0.00	-0.00	0.00	0.00
2	1	0.40	0.00	0.00	0.00	-0.00	0.00	0.00
2	1	0.60	0.00	0.00	0.00	-0.00	0.00	0.00
2	1	0.80	0.00	0.00	0.00	-0.00	0.00	0.00
2	1	1.00	0.00	0.00	0.00	-0.00	0.00	0.00

Lc	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
3	1	0.00	0.00	0.00	0.00	19897.84	0.00	0.00
3	1	0.20	0.00	0.00	0.00	19897.84	0.00	0.00
3	1	0.40	0.00	0.00	0.00	19897.84	0.00	0.00
3	1	0.60	0.00	0.00	0.00	19897.84	0.00	0.00
3	1	0.80	0.00	0.00	0.00	19897.84	0.00	0.00
3	1	1.00	0.00	0.00	0.00	19897.84	0.00	0.00

Figure 5.3 Listing of results

Results:

Displacement at node 2 (tip of cantilever):

Loadcase 1: Immediate displacement when framework is removed: 20.3 mm.

Loadcase 2: Creep before tip is connected: 11.1 mm.

Loadcase 3: Creep after tip is connected: 5.6 mm.

Total displacement after 9999 days: 36.9 mm.

Redistribution of forces starting when cantilever is connected, is found from analysis 3 and loadcase 3

At support the moment is $-120000 + 19898 = -100102$ kNm.

At field/tip the moment is 19898 kNm.

Using the simplified 22/78 method gives:

$$M = 0.22 * M_0 + 0.78 * M_{\infty}$$

$$\text{Support; } 0.22 * 120000 + 0.78 * 80000 = 88800 \text{ kNm}$$

$$\text{Field; } 0.22 * 0.0 + 0.78 * 40000 = 31200 \text{ kNm}$$

6 Creep loads: Creep-2

This example shows the use of multiple analyses and calculation of creep.

Example folder: *Creep-2*
 Example files: *Creep-2.inp*
Creep-2.lod
Creep-2.mod
Creep-2.cmb

The example is obtained from: “Trost et al., Der Bauingenieur, 1970, Heft 5”.

The system and its different construction stages is shown in figure 6.1

A continuous beam with three spans is cast in three steps. Each stage in the process has its own model.

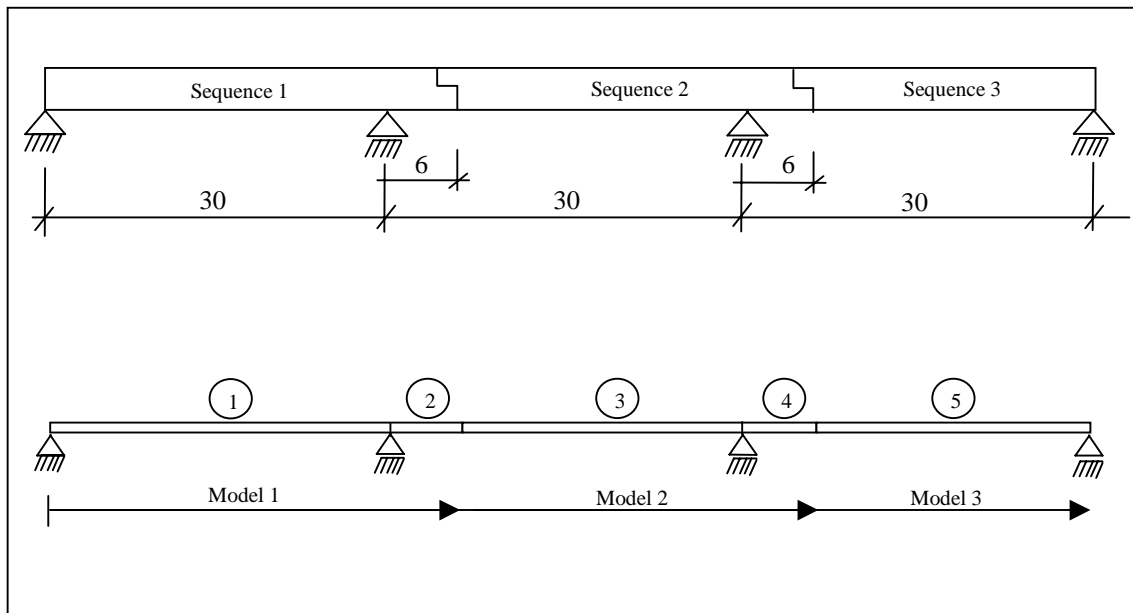


Figure 6.1 Construction stages, system and sub models

Each sequence is an addition to the existing model resulting in a new model.

The building process can be represented with 5 different stages to be analysed see fig 1.6.2.

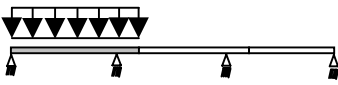
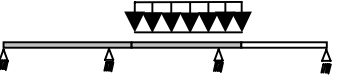



Loadcase	Model	Analysis/phase
1		Sequence 1, Model 1 Analysis 1 T = 0 – 7 [days]
2		Sequence 2, Model 2 Analysis 2 T = 14 – 21 [days]
3		Creep redistribution, Model 2 Analysis 10 T = 21 – 35 [days]
4		Sequence 3, Model 3 Analysis 3 T = 28 – 35 [days]
5		Creep redistribution, Model 3 Analysis 11 T = 35 – ∞ [days]

Figure 6.2 Analyses

The input is listed below:

```

Project name: Creep 2

*** NODE INPUT ***
NODEINS 1 1 1 0.000 0.000 0.000 0.000 0.000 0.000
NODEINS 2 2 1 0.000 30.000 0.000 0.000 0.000 0.000
NODEINS 3 3 1 0.000 36.000 0.000 0.000 0.000 0.000
NODEINS 4 4 1 0.000 60.000 0.000 0.000 0.000 0.000
NODEINS 5 5 1 0.000 66.000 0.000 0.000 0.000 0.000
NODEINS 6 6 1 0.000 90.000 0.000 0.000 0.000 0.000

*** ELEMENT INPUT ***
ELEMINS 1 5 1 1 2 1

*** GEOMETRY SECTION INPUT ***
SECTINS 1 1 30000 0.2 1.000e+001 1.000e+001 1.000e+001 1.000e+001

*** ELEMENT SPECIFICATION INPUT ***
ELSPINS 1 5 1 1 0 4

*** BOUNDARY CONDITION INPUT ***
BOUNDINS 1 1 1 1 1 1 0 1 1
BOUNDINS 2 2 1 1 0 1 0 1 1
BOUNDINS 4 4 1 1 0 1 0 1 1
BOUNDINS 6 6 1 1 0 1 0 1 1

*** JOINT INPUT ***

*** DESIGN SECTION INPUT ***
DESGINS 1 1 1 0.000 1.000 0.200
DESGINS 2 2 1 0.000 1.000 1.000
DESGINS 3 3 1 0.000 1.000 0.250
DESGINS 4 4 1 0.000 1.000 1.000
DESGINS 5 5 1 0.000 1.000 0.250

*** LOAD INPUT ***
LOADINS 1 1 2 1 3 3 -100.000 0.000 0.000 0.000
LOADINS 2 3 4 1 3 3 -100.000 0.000 0.000 0.000
LOADINS 4 5 5 1 3 3 -100.000 0.000 0.000 0.000
LOADINS 10 1 5 1 3 3 -100.000 0.000 0.000 0.000

*** CREEP COMBINATIONS ***
CREEPCMB 1 1 1.000 2 1.000

```

```

CREEPCMB 2 1 1.000 2 1.000 3 1.000
CREEPCMB 2 4 1.000

*** CREEP LOADCASES ***
CREEPLC 3 1 2 1 1 2 0.170 0.000 0.000 Kryp
CREEPLC 3 3 4 1 1 2 0.480 0.000 0.000 Kryp
CREEPLC 5 1 2 1 2 2 1.030 0.000 0.000 Kryp
CREEPLC 5 3 4 1 2 2 1.340 0.000 0.000 Kryp
CREEPLC 5 5 5 1 2 2 2.210 0.000 0.000 Kryp

*** SUBMODELS INPUT ***
MODEL 1 -1 3 1 2 0
MODEL 2 1 3 3 4 0
MODEL 3 2 3 5 5 0

*** SUBMODEL MODIFICATION INPUT ***

*** CALCULATION GROUP INPUT ***
CALCGRP 1 1 1 1 1
CALCGRP 2 1 2 2 1
CALCGRP 3 1 4 4 1
CALCGRP 4 1 10 10 1
CALCGRP 10 3 3 3 1
CALCGRP 11 3 5 5 1

*** ANALYSES INPUT ***
ANALYSIS 1 1 1 0 0.000 Self weight cast 1
ANALYSIS 2 2 2 0 0.000 Self weight cast 2
ANALYSIS 3 3 3 0 0.000 Self weight cast 3
ANALYSIS 4 3 4 0 0.000 Self weight cast entire model
ANALYSIS 10 2 10 0 0.000 Creep part 1
ANALYSIS 11 3 11 0 0.000 Creep part 2

*** ORDINARY COMBINATION INPUT ***
ORDCOMB 6 1 1 1 1.000 2 1.000 4 1.000
ORDCOMB 10 1 1 1 1.000 2 1.000 3 1.000
ORDCOMB 10 1 1 4 1.000 5 1.000
ORDCOMB 11 1 1 10 0.780
ORDCOMB 11 1 2 6 0.220

```

Figure 6.3 ASCII input (all files).

Combined results are shown in figure 1.6.4. Combination 10 is the sum of loadcase 1-5, which gives the force at infinite time. Combination 11 is calculated by the 22/78 method.

The results are compared with moments at support calculated by “Troost et al.” in figure 1.6.5. The difference between Combination 10 and the result from “Troost” is mainly the difference in partial creep factors.

Comb	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
10	1	0.00	0.00	0.00	-1255.21	-0.00	0.00	0.00
10	1	0.20	0.00	0.00	-655.21	5731.27	0.00	0.00
10	1	0.40	0.00	0.00	-55.21	7862.55	0.00	0.00
10	1	0.60	0.00	0.00	544.79	6393.82	0.00	0.00
10	1	0.80	0.00	0.00	1144.79	1325.10	0.00	0.00
10	1	1.00	0.00	0.00	1744.79	-7343.63	0.00	0.00
10	2	0.00	0.00	0.00	-1443.31	-7343.33	0.00	0.00
10	2	1.00	0.00	0.00	-843.31	-483.45	0.00	0.00
10	3	0.00	0.00	0.00	-843.31	-483.39	0.00	0.00
10	3	0.25	0.00	0.00	-243.31	2776.48	0.00	0.00
10	3	0.50	0.00	0.00	356.69	2436.36	0.00	0.00
10	3	0.75	0.00	0.00	956.69	-1503.76	0.00	0.00
10	3	1.00	0.00	0.00	1556.69	-9043.89	0.00	0.00
10	4	0.00	0.00	0.00	-1801.44	-9043.65	0.00	0.00
10	4	1.00	0.00	0.00	-1201.44	-34.98	0.00	0.00
10	5	0.00	0.00	0.00	-1201.44	-34.92	0.00	0.00

10	5	0.25	0.00	0.00	-601.44	5373.75	0.00	0.00
10	5	0.50	0.00	0.00	-1.44	7182.42	0.00	0.00
10	5	0.75	0.00	0.00	598.56	5391.09	0.00	0.00
10	5	1.00	0.00	0.00	1198.56	-0.24	0.00	0.00
Comb	Elem	X/L	Axial [kN]	Shear L [kN]	Shear N [kN]	Bending L [kNm]	Bending N [kNm]	Torsion [kNm]
11	1	0.00	0.00	0.00	-1229.76	-0.00	0.00	0.00
11	1	0.20	0.00	0.00	-629.76	5578.56	0.00	0.00
11	1	0.40	0.00	0.00	-29.76	7557.12	0.00	0.00
11	1	0.60	0.00	0.00	570.24	5935.69	0.00	0.00
11	1	0.80	0.00	0.00	1170.24	714.25	0.00	0.00
11	1	1.00	0.00	0.00	1770.24	-8107.19	0.00	0.00
11	2	0.00	0.00	0.00	-1484.72	-8106.89	0.00	0.00
11	2	1.00	0.00	0.00	-884.72	-998.56	0.00	0.00
11	3	0.00	0.00	0.00	-884.72	-998.50	0.00	0.00
11	3	0.25	0.00	0.00	-284.72	2509.84	0.00	0.00
11	3	0.50	0.00	0.00	315.28	2418.17	0.00	0.00
11	3	0.75	0.00	0.00	915.28	-1273.50	0.00	0.00
11	3	1.00	0.00	0.00	1515.28	-8565.17	0.00	0.00
11	4	0.00	0.00	0.00	-1785.49	-8564.93	0.00	0.00
11	4	1.00	0.00	0.00	-1185.49	348.00	0.00	0.00
11	5	0.00	0.00	0.00	-1185.49	348.06	0.00	0.00
11	5	0.25	0.00	0.00	-585.49	5660.98	0.00	0.00
11	5	0.50	0.00	0.00	14.51	7373.91	0.00	0.00
11	5	0.75	0.00	0.00	614.51	5486.83	0.00	0.00
11	5	1.00	0.00	0.00	1214.51	-0.24	0.00	0.00

Figure 6.4 Results of combinations.

Section	Comb 10	Comb 11 (0.22*M ₀ + 0.78*M _∞)	Trost
El 1 – 1.00	-7344	-8107	-7891
El 3 – 1.00	-9044	-8565	-8911

Figure 6.5 Comparison of results, Moment ML [kNm].

7 Creep loads: Creep-3

This example shows how a creep calculation for a cantilever with stressed tendons is handled.

Example folder: *Creep-3*
 Example files: *Creep-3.inp*
Creep-3 lod
Creep-3.mod
Creep-3.cmb

The model and the amount of tendons are shown on figure 7.1.

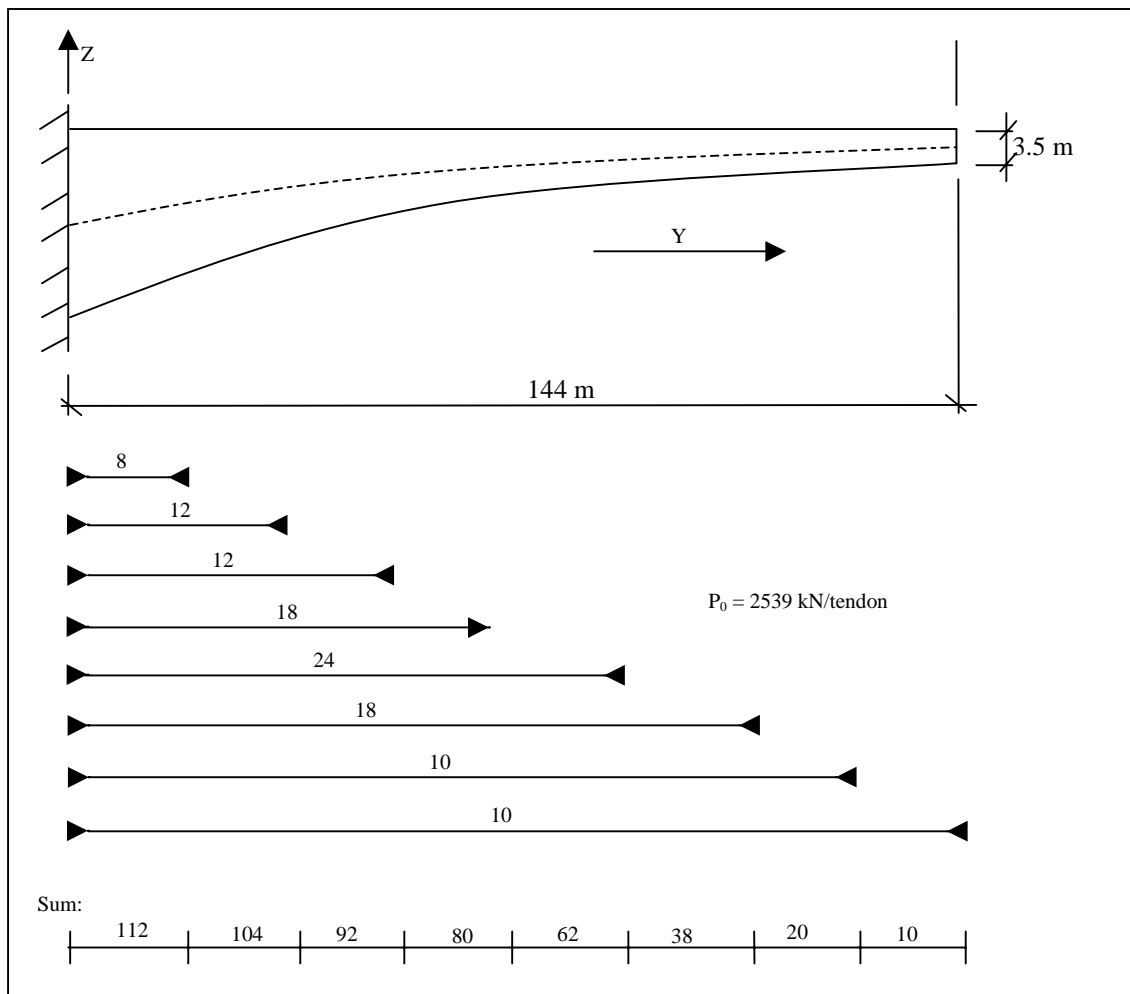


Figure 7.1 Geometry and prestressing cables

In this calculation the redistribution of forces due to creep is calculated. Effects of creep before the connection has no influence on the force distribution. Partial creep factors are calculated according to:

Elements:	1-4	5-8	9-12	13-16	17-20	21-24	25-28	29-32
$H_y[\text{mm}]$	13000	10000	8200	6600	5100	4200	3700	3500
$H_i[\text{mm}]$	11500	8500	6700	5100	3600	2700	2200	2000
Age: t_i	250	200	160	125	100	70	40	14
$\Delta\phi$	0.79	0.85	0.91	0.98	1.03	1.11	1.23	1.41

$t_0 = 3$ days
 $t_\infty = 9999$ days
 $RH = 70\%$
 $f_{ck} = 54$ Mpa (C65)

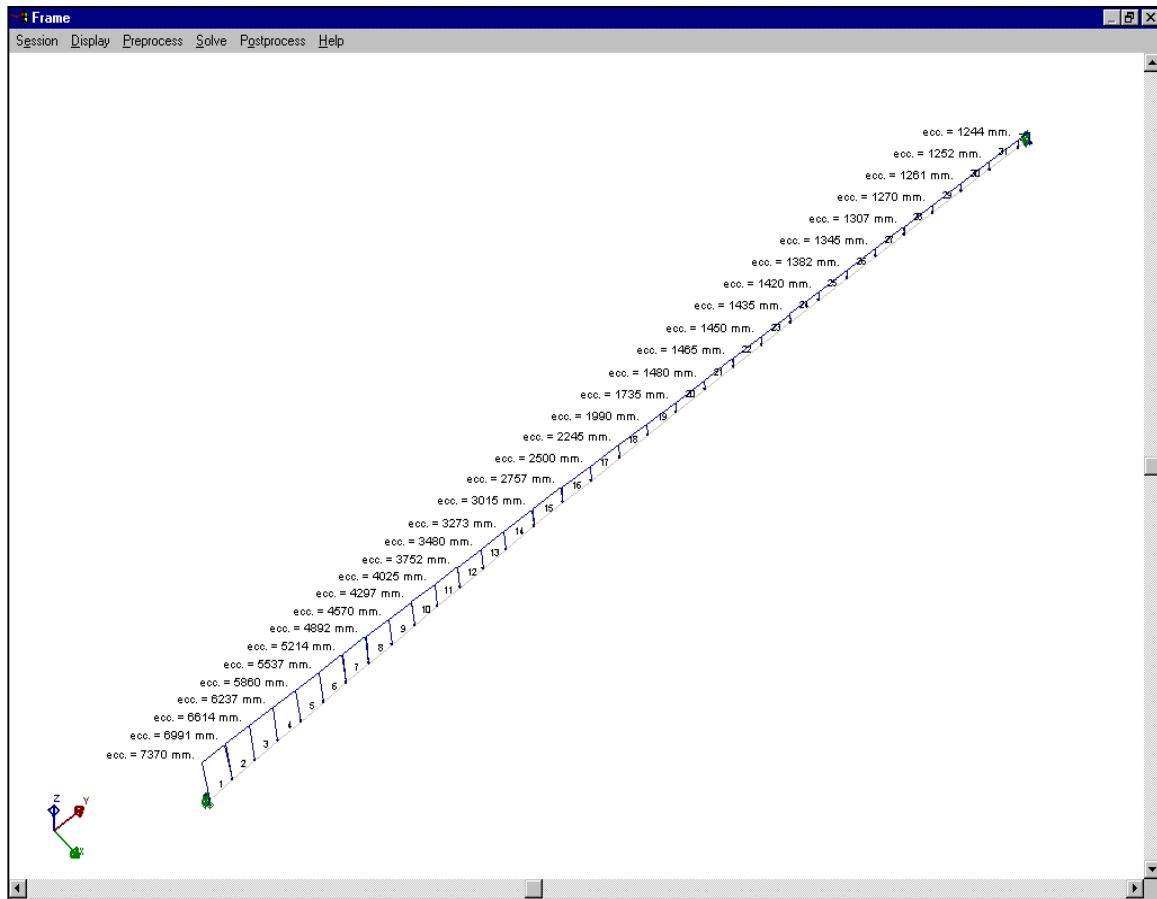
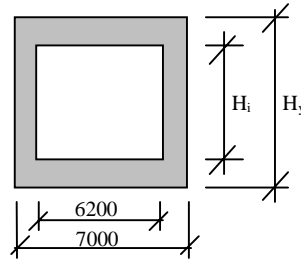


Figure 7.2 Eccentricities for tendons, element numbers are also shown.

Two static models are used:

- Model 1 is a cantilever used for calculating section forces for self-weight and prestressing prior to connection. This model consists of element 1 to 31.
- Model 2 also includes element 32 and constraints applicable for the mid-field.

Please note that both the left and the right end of model 2 is restrained in global Y-direction. Due to creep effects following connection one can expect contribution from tension in the cantilever from the creep phase.

The analyses are performed in three steps:

Analysis 1; Before the cantilever is connected. Ordinary static loads are:

Self weight loadcase 11.

Prestress (parasite) loadcase 12. (Used for design).

Total section forces due to prestress loadcase 13. (For the creep analysis).

Analysis 2; Cantilever is connected. A creep analysis is performed for:
Creep loadcase 14.

Analysis 3; Cantilever is connected. Ordinary static loads are:
Self weight loadcase 21.
Prestress (parasite) loadcase 22.

The loadcases 21 and 22 are used to estimate the effect of creep based on a simplified method where 22% of the initial state is added to 78% of the final state.

The input required to establish geometry, loads, models and analyses are shown below. In addition the definition of a few load combinations are included. See figure 7.3 - 7.6.

The combined results of the analyses are shown in figure 7.7. The results at support (Element 1 $X/L = 0.0$) and at mid-field (Element 32 $X/L = 1.0$) is presented. Combination 1- 7 are the result of each loadcase. Note that prestress loadcase 12 (comb.2) does not give any section forces while prestress loadcase 13 (comb.3) gives the total forces.

The redistribution of creep is stored to loadcase 14 (comb.4). Note that there is a considerable tension in the cantilever due to the constraints in both ends.

Combination 7 gives the total section forces after creep. These forces can be compared with section forces for self-weight Lc 11 before connection. Note that the parasite forces due to prestressing is zero.

Normally one would expect that the section forces at support should decrease and that the section forces in the mid-field should increase. The situation for this example is that the section force at support also increases relative to the moment obtained from a free cantilever. The reason for this is the curvature of the cantilever COG, see fig 1.7.1, and the axial tension due to creep redistribution.

Combination 8 presents the total section forces by using the simplified 22/78 method. The results compare reasonably well. This method also provides axial tension and eccentricity moments.

Project name: Creep-3								
*** NODE INPUT ***								
NODEINS	101	0	0	0	0	44.36	0	0
NODEINS	102	0	0	0	4	44.97	0	0
NODEINS	103	0	0	0	8	45.46	0	0
NODEINS	104	0	0	0	12	45.93	0	0
NODEINS	105	0	0	0	16	46.41	0	0
NODEINS	106	0	0	0	20	46.88	0	0
NODEINS	107	0	0	0	24	47.32	0	0
NODEINS	108	0	0	0	28	47.71	0	0
NODEINS	109	0	0	0	32	48.06	0	0
NODEINS	110	0	0	0	36	48.45	0	0
NODEINS	111	0	0	0	40	48.82	0	0
NODEINS	112	0	0	0	44	49.17	0	0
NODEINS	113	0	0	0	48	49.53	0	0
NODEINS	114	0	0	0	52	49.86	0	0
NODEINS	115	0	0	0	57	50.31	0	0
NODEINS	116	0	0	0	62	50.62	0	0
NODEINS	117	0	0	0	67	50.73	0	0
NODEINS	118	0	0	0	72	51.02	0	0
NODEINS	119	0	0	0	77	51.29	0	0
NODEINS	120	0	0	0	82	51.54	0	0
NODEINS	121	0	0	0	87	51.77	0	0
NODEINS	122	0	0	0	92	51.98	0	0
NODEINS	123	0	0	0	97	52.06	0	0
NODEINS	124	0	0	0	102	52.24	0	0
NODEINS	125	0	0	0	107	52.35	0	0
NODEINS	126	0	0	0	112	52.45	0	0
NODEINS	127	0	0	0	117	52.49	0	0
NODEINS	128	0	0	0	122	52.56	0	0
NODEINS	129	0	0	0	127	52.6	0	0
NODEINS	130	0	0	0	132	52.64	0	0
NODEINS	131	0	0	0	137	52.65	0	0
NODEINS	132	0	0	0	142	52.66	0	0
NODEINS	133	0	0	0	144	52.66	0	0
*** ELEMENT INPUT ***								
ELEMINS	1	32	1	101	102	1		
*** GEOMETRY SECTION INPUT ***								
SECTINS	1	1	29500.0	0.0	22.20	652.40	355.70	176.80
SECTINS	2	1	29500.0	0.0	21.60	572.80	331.10	170.30
SECTINS	3	1	29500.0	0.0	21.00	502.50	307.70	164.10
SECTINS	4	1	29500.0	0.0	20.50	440.30	285.50	158.20
SECTINS	5	1	29500.0	0.0	19.80	383.20	264.30	152.10
SECTINS	6	1	29500.0	0.0	19.20	332.60	244.30	146.20
SECTINS	7	1	29500.0	0.0	18.60	289.00	225.40	140.70
SECTINS	8	1	29500.0	0.0	18.10	252.70	208.00	135.90
SECTINS	9	1	29500.0	0.0	17.00	217.40	176.20	124.10
SECTINS	10	1	29500.0	0.0	16.50	188.90	162.90	119.90
SECTINS	11	1	29500.0	0.0	16.00	163.70	149.80	115.60
SECTINS	12	1	29500.0	0.0	15.50	141.80	137.50	111.50
SECTINS	13	1	29500.0	0.0	14.80	121.80	126.00	107.20
SECTINS	14	1	29500.0	0.0	14.30	103.80	114.20	103.20
SECTINS	15	1	29500.0	0.0	13.30	84.10	101.30	97.40
SECTINS	16	1	29500.0	0.0	12.80	71.20	90.80	93.80
SECTINS	17	1	29500.0	0.0	11.80	57.60	78.00	88.70
SECTINS	18	1	29500.0	0.0	11.30	48.90	70.00	85.30
SECTINS	19	1	29500.0	0.0	10.80	41.60	62.80	82.20
SECTINS	20	1	29500.0	0.0	10.40	35.70	56.50	79.40
SECTINS	21	1	29500.0	0.0	9.90	30.60	50.80	76.60
SECTINS	22	1	29500.0	0.0	9.50	26.60	46.00	74.20
SECTINS	23	1	29500.0	0.0	8.10	21.60	36.00	67.50
SECTINS	24	1	29500.0	0.0	7.70	19.00	32.90	65.70
SECTINS	25	1	29500.0	0.0	7.60	17.50	30.90	64.60
SECTINS	26	1	29500.0	0.0	7.50	16.30	29.30	63.80
SECTINS	27	1	29500.0	0.0	7.20	15.30	28.00	59.30
SECTINS	28	1	29500.0	0.0	7.10	14.50	26.90	58.60
SECTINS	29	1	29500.0	0.0	7.00	14.10	26.30	58.30
SECTINS	30	1	29500.0	0.0	7.00	13.80	25.90	58.10

SECTINS	31	1	29500.0	0.0	7.00	13.80	25.80	58.00
SECTINS	32	1	29500.0	0.0	7.00	13.70	25.80	58.00
*** ELEMENT SPECIFICATION INPUT ***								
ELSPINS	1	32	1	1	1	4		
*** BOUNDARY CONDITION INPUT ***								
BOUNDINS	101		0	0	1	1	1	1
BOUNDINS	133		0	0	1	1	0	1
*** DESIGN SECTION INPUT ***								
DESGINS	1	31		1	0	1	2	
DESGINS	32	32		1	0	1	1	

Figure 7.3 Geometry input

Project name: Creep-3									
*** LOAD INPUT ***									
LOADINS	11	1	31	1	1	3	-26.0	0.000	0.000 0.0 SelfWeight 1-31
LOADINS	21	1	32	1	1	3	-26.0	0.000	0.000 0.0 SelfWight 32
LOADINS	12	1	1	1	7	5	2539.0	7.370	-0.379 112.0 Prest. parasitel
LOADINS	12	2	2	1	7	5	2538.0	6.991	-0.377 112.0 Prest. parasitel
LOADINS	12	3	3	1	7	5	2538.0	6.614	-0.377 112.0 Prest. parasitel
LOADINS	12	4	4	1	7	5	2539.0	6.237	-0.377 112.0 Prest. parasitel
LOADINS	12	5	5	1	7	5	2539.0	5.860	-0.323 104.0 Prest. parasitel
LOADINS	12	6	6	1	7	5	2538.0	5.537	-0.323 104.0 Prest. parasitel
LOADINS	12	7	7	1	7	5	2538.0	5.214	-0.322 104.0 Prest. parasitel
LOADINS	12	8	8	1	7	5	2539.0	4.892	-0.322 104.0 Prest. parasitel
LOADINS	12	9	9	1	7	5	2539.0	4.570	-0.273 92.0 Prest. parasitel
LOADINS	12	10	10	1	7	5	2538.0	4.297	-0.273 92.0 Prest. parasitel
LOADINS	12	11	11	1	7	5	2538.0	4.025	-0.272 92.0 Prest. parasitel
LOADINS	12	12	12	1	7	5	2539.0	3.752	-0.272 92.0 Prest. parasitel
LOADINS	12	13	13	1	7	5	2539.0	3.480	-0.207 80.0 Prest. parasitel
LOADINS	12	14	14	1	7	5	2538.0	3.273	-0.258 80.0 Prest. parasitel
LOADINS	12	15	15	1	7	5	2538.0	3.015	-0.258 80.0 Prest. parasitel
LOADINS	12	16	16	1	7	5	2539.0	2.757	-0.257 80.0 Prest. parasitel
LOADINS	12	17	17	1	7	5	2539.0	2.500	-0.255 62.0 Prest. parasitel
LOADINS	12	18	18	1	7	5	2538.0	2.245	-0.255 62.0 Prest. parasitel
LOADINS	12	19	19	1	7	5	2538.0	1.990	-0.255 62.0 Prest. parasitel
LOADINS	12	20	20	1	7	5	2539.0	1.735	-0.255 62.0 Prest. parasitel
LOADINS	12	21	21	1	7	5	2539.0	1.480	-0.015 38.0 Prest. parasitel
LOADINS	12	22	22	1	7	5	2538.0	1.465	-0.015 38.0 Prest. parasitel
LOADINS	12	23	23	1	7	5	2538.0	1.450	-0.015 38.0 Prest. parasitel
LOADINS	12	24	24	1	7	5	2539.0	1.435	-0.015 38.0 Prest. parasitel
LOADINS	12	25	25	1	7	5	2539.0	1.420	-0.038 20.0 Prest. parasitel
LOADINS	12	26	26	1	7	5	2538.0	1.382	-0.037 20.0 Prest. parasitel
LOADINS	12	27	27	1	7	5	2538.0	1.345	-0.038 20.0 Prest. parasitel
LOADINS	12	28	28	1	7	5	2539.0	1.307	-0.037 20.0 Prest. parasitel
LOADINS	12	29	29	1	7	5	2539.0	1.270	-0.009 10.0 Prest. parasitel
LOADINS	12	30	30	1	7	5	2538.0	1.261	-0.009 10.0 Prest. parasitel
LOADINS	12	31	31	1	7	5	2539.0	1.252	-0.009 10.0 Prest. parasitel
LOADINS	12	32	32	1	7	5	2540.0	1.244	-0.004 10.0 Prest. parasitel
LOADINS	13	1	1	1	11	5	2539.0	7.370	-0.379 112.0 Prest. creep
LOADINS	13	2	2	1	11	5	2538.0	6.991	-0.377 112.0 Prest. creep
LOADINS	13	3	3	1	11	5	2538.0	6.614	-0.377 112.0 Prest. creep
LOADINS	13	4	4	1	11	5	2539.0	6.237	-0.377 112.0 Prest. creep
LOADINS	13	5	5	1	11	5	2539.0	5.860	-0.323 104.0 Prest. creep
LOADINS	13	6	6	1	11	5	2538.0	5.537	-0.323 104.0 Prest. creep
LOADINS	13	7	7	1	11	5	2538.0	5.214	-0.322 104.0 Prest. creep
LOADINS	13	8	8	1	11	5	2539.0	4.892	-0.322 104.0 Prest. creep
LOADINS	13	9	9	1	11	5	2539.0	4.570	-0.273 92.0 Prest. creep
LOADINS	13	10	10	1	11	5	2538.0	4.297	-0.273 92.0 Prest. creep
LOADINS	13	11	11	1	11	5	2538.0	4.025	-0.272 92.0 Prest. creep
LOADINS	13	12	12	1	11	5	2539.0	3.752	-0.272 92.0 Prest. creep
LOADINS	13	13	13	1	11	5	2539.0	3.480	-0.207 80.0 Prest. creep
LOADINS	13	14	14	1	11	5	2538.0	3.273	-0.258 80.0 Prest. creep
LOADINS	13	15	15	1	11	5	2538.0	3.015	-0.258 80.0 Prest. creep
LOADINS	13	16	16	1	11	5	2539.0	2.757	-0.257 80.0 Prest. creep
LOADINS	13	17	17	1	11	5	2539.0	2.500	-0.255 62.0 Prest. creep

LOADINS	13	18	18	1	11	5	2538.0	2.245	-0.255	62.0	Prest. creep
LOADINS	13	19	19	1	11	5	2538.0	1.990	-0.255	62.0	Prest. creep
LOADINS	13	20	20	1	11	5	2539.0	1.735	-0.255	62.0	Prest. creep
LOADINS	13	21	21	1	11	5	2539.0	1.480	-0.015	38.0	Prest. creep
LOADINS	13	22	22	1	11	5	2538.0	1.465	-0.015	38.0	Prest. creep
LOADINS	13	23	23	1	11	5	2538.0	1.450	-0.015	38.0	Prest. creep
LOADINS	13	24	24	1	11	5	2539.0	1.435	-0.015	38.0	Prest. creep
LOADINS	13	25	25	1	11	5	2539.0	1.420	-0.038	20.0	Prest. creep
LOADINS	13	26	26	1	11	5	2538.0	1.382	-0.037	20.0	Prest. creep
LOADINS	13	27	27	1	11	5	2538.0	1.345	-0.038	20.0	Prest. creep
LOADINS	13	28	28	1	11	5	2539.0	1.307	-0.037	20.0	Prest. creep
LOADINS	13	29	29	1	11	5	2539.0	1.270	-0.009	10.0	Prest. creep
LOADINS	13	30	30	1	11	5	2538.0	1.261	-0.009	10.0	Prest. creep
LOADINS	13	31	31	1	11	5	2539.0	1.252	-0.009	10.0	Prest. creep
LOADINS	13	32	32	1	11	5	2540.0	1.244	-0.004	10.0	Prest. creep
LOADINS	22	1	1	1	7	5	2539.0	7.370	-0.379	112.0	Prest. parasite2
LOADINS	22	2	2	1	7	5	2538.0	6.991	-0.377	112.0	Prest. parasite2
LOADINS	22	3	3	1	7	5	2538.0	6.614	-0.377	112.0	Prest. parasite2
LOADINS	22	4	4	1	7	5	2539.0	6.237	-0.377	112.0	Prest. parasite2
LOADINS	22	5	5	1	7	5	2539.0	5.860	-0.323	104.0	Prest. parasite2
LOADINS	22	6	6	1	7	5	2538.0	5.537	-0.323	104.0	Prest. parasite2
LOADINS	22	7	7	1	7	5	2538.0	5.214	-0.322	104.0	Prest. parasite2
LOADINS	22	8	8	1	7	5	2539.0	4.892	-0.322	104.0	Prest. parasite2
LOADINS	22	9	9	1	7	5	2539.0	4.570	-0.273	92.0	Prest. parasite2
LOADINS	22	10	10	1	7	5	2538.0	4.297	-0.273	92.0	Prest. parasite2
LOADINS	22	11	11	1	7	5	2538.0	4.025	-0.272	92.0	Prest. parasite2
LOADINS	22	12	12	1	7	5	2539.0	3.752	-0.272	92.0	Prest. parasite2
LOADINS	22	13	13	1	7	5	2539.0	3.480	-0.207	80.0	Prest. parasite2
LOADINS	22	14	14	1	7	5	2538.0	3.273	-0.258	80.0	Prest. parasite2
LOADINS	22	15	15	1	7	5	2538.0	3.015	-0.258	80.0	Prest. parasite2
LOADINS	22	16	16	1	7	5	2539.0	2.757	-0.257	80.0	Prest. parasite2
LOADINS	22	17	17	1	7	5	2539.0	2.500	-0.255	62.0	Prest. parasite2
LOADINS	22	18	18	1	7	5	2538.0	2.245	-0.255	62.0	Prest. parasite2
LOADINS	22	19	19	1	7	5	2538.0	1.990	-0.255	62.0	Prest. parasite2
LOADINS	22	20	20	1	7	5	2539.0	1.735	-0.255	62.0	Prest. parasite2
LOADINS	22	21	21	1	7	5	2539.0	1.480	-0.015	38.0	Prest. parasite2
LOADINS	22	22	22	1	7	5	2538.0	1.465	-0.015	38.0	Prest. parasite2
LOADINS	22	23	23	1	7	5	2538.0	1.450	-0.015	38.0	Prest. parasite2
LOADINS	22	24	24	1	7	5	2539.0	1.435	-0.015	38.0	Prest. parasite2
LOADINS	22	25	25	1	7	5	2539.0	1.420	-0.038	20.0	Prest. parasite2
LOADINS	22	26	26	1	7	5	2538.0	1.382	-0.037	20.0	Prest. parasite2
LOADINS	22	27	27	1	7	5	2538.0	1.345	-0.038	20.0	Prest. parasite2
LOADINS	22	28	28	1	7	5	2539.0	1.307	-0.037	20.0	Prest. parasite2
LOADINS	22	29	29	1	7	5	2539.0	1.270	-0.009	10.0	Prest. parasite2
LOADINS	22	30	30	1	7	5	2538.0	1.261	-0.009	10.0	Prest. parasite2
LOADINS	22	31	31	1	7	5	2539.0	1.252	-0.009	10.0	Prest. parasite2
LOADINS	22	32	32	1	7	5	2540.0	1.244	-0.004	10.0	Prest. parasite2
*** CREEP LOAD INPUT ***											
CREEPCMB	1			11	1.00		13	1.00	0	0.000	Creep combination 1
*** CREEP COMBINATION INPUT ***											
CREEPLC	14	1	4	1	1	2	0.79	0.000	0.000	Creep Lc14	
CREEPLC	14	5	8	1	1	2	0.85	0.000	0.000	Creep Lc14	
CREEPLC	14	9	12	1	1	2	0.91	0.000	0.000	Creep Lc14	
CREEPLC	14	13	16	1	1	2	0.98	0.000	0.000	Creep Lc14	
CREEPLC	14	17	20	1	1	2	1.03	0.000	0.000	Creep Lc14	
CREEPLC	14	21	24	1	1	2	1.11	0.000	0.000	Creep Lc14	
CREEPLC	14	25	28	1	1	2	1.23	0.000	0.000	Creep Lc14	
CREEPLC	14	29	32	1	1	2	1.41	0.000	0.000	Creep Lc14	

Figure 7.4 Load input

```

Project name: Creep-3

*** SUBMODELS INPUT ***
MODEL      1      -1      3      1      31      1 Canteliver
MODEL      2      -1      3      1      32      1 Connected

*** SUBMODEL MODIFICATION INPUT ***      *** CALCULATION GROUP INPUT ***
CALCGRP     1      1      11      13      1 Static build
CALCGRP     2      3      14      14      1 Creep load
CALCGRP     3      1      21      22      1 Static finished

*** ANALYSES INPUT ***
ANALYSIS    1      1      1      0      0.00 Canteliver static
ANALYSIS    2      2      2      0      0.00 Connected creep
ANALYSIS    3      2      3      0      0.00 Connected static

```

Figure 7.5 Models and analysis input

```

Project name: Creep-3

ORDCOMB  1  1  1  11      1      0      0      0      0 Loadcase 11
ORDCOMB  2  1  1  12      1      0      0      0      0 Loadcase 12
ORDCOMB  3  1  1  13      1      0      0      0      0 Loadcase 13
ORDCOMB  4  1  1  14      1      0      0      0      0 Loadcase 14
ORDCOMB  5  1  1  21      1      0      0      0      0 Loadcase 21
ORDCOMB  6  1  1  22      1      0      0      0      0 Loadcase 22
ORDCOMB  7  1  1  11      1      12      1      14      1 Total w/creep
ORDCOMB  8  1  1  11      0.22  12      0.22  21      0.78 Total Simplified
ORDCOMB  8  1  1  22      0.78

```

Figure 7.6 Load combination input

Elem	X/L	Comb	[kN] Axial	[kN] Shear L	[kN] Shear N	[kNm] Bending L	[kNm] Bending N	[kNm] Torsion
1	0.00	1	-7138.96	0.00	-46812.86	-2616716.12	0.00	0.00
1	0.00	2	-0.00	0.00	-0.00	-0.00	0.00	0.00
1	0.00	3	-283128.69	0.00	26519.84	2086655.82	0.00	0.00
1	0.00	4	37353.38	0.00	-5696.39	-273433.63	0.00	0.00
1	0.00	5	-92528.05	0.00	-34159.23	-1795108.42	0.00	0.00
1	0.00	6	155676.50	0.00	-23740.67	-1380359.11	0.00	0.00
1	0.00	7	30214.42	0.00	-52509.25	-2890149.75	0.00	0.00
1	0.00	8	47685.22	0.00	-55460.75	-3052542.22	0.00	0.00
32	1.00	4	37785.23	0.00	-0.00	40183.77	0.00	0.00
32	1.00	5	-86320.78	0.00	-0.00	157197.16	0.00	0.00
32	1.00	6	157476.32	0.00	-0.00	-73305.65	0.00	0.00
32	1.00	7	37785.23	0.00	-0.00	40183.77	0.00	0.00
32	1.00	8	55501.32	0.00	-0.00	65435.38	0.00	0.00

Figure 7.7 Results

